

# CONTINUUM STRUCTURAL TOPOLOGY OPTIMIZATION

**Colby C. Swan**

Department of Civil and Environmental Engineering  
Center for Computer-Aided Design  
The University of Iowa

CEE-SMM Graduate Seminar  
31 March 2006



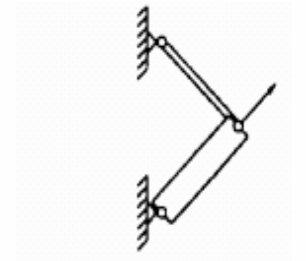
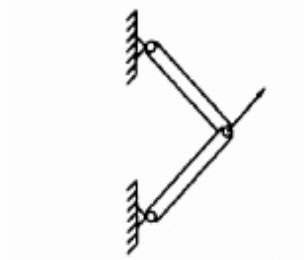
“Continuum Structural Topology Optimization” presented to CEE/SMM Seminar, 31 March 2006



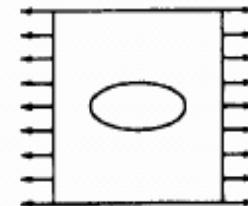
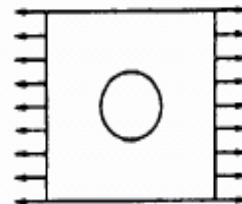
# Introduction

## Structural Optimization

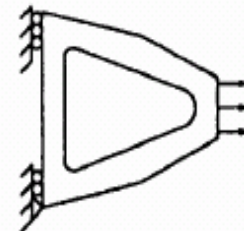
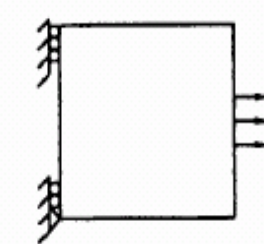
. Size Optimization



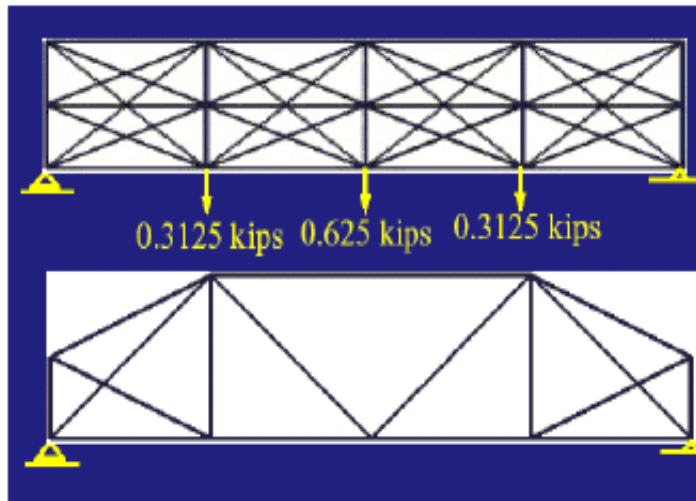
. Shape Optimization



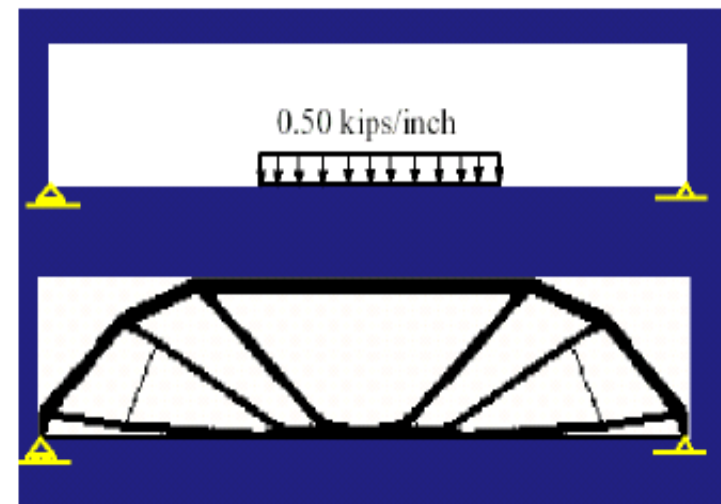
. Topology Optimization



# Alternative Approaches to Structural Topology Optimization



**Discrete structural model of beam/truss elements**



**Continuum structural model**

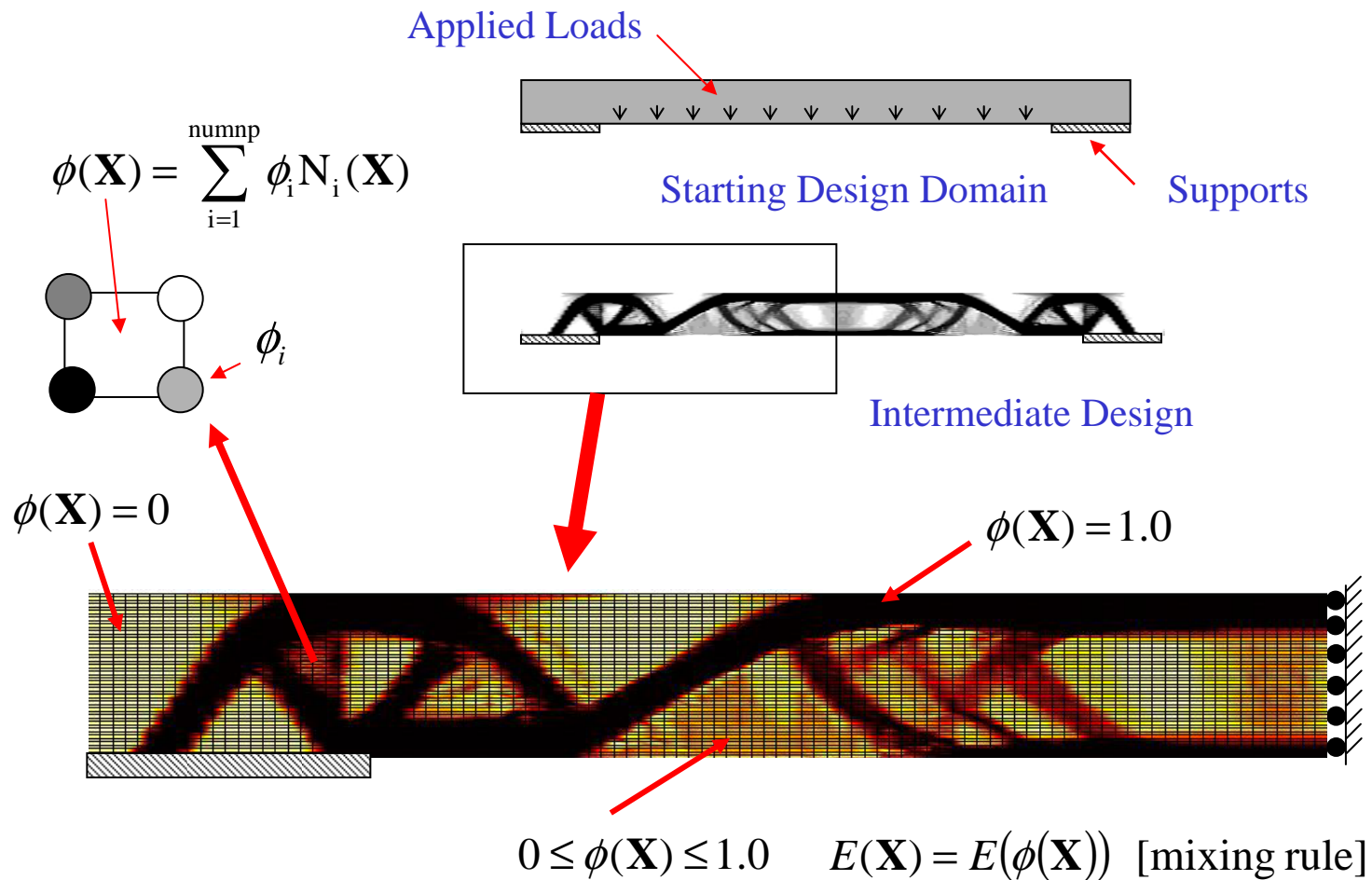


## OUTLINE:

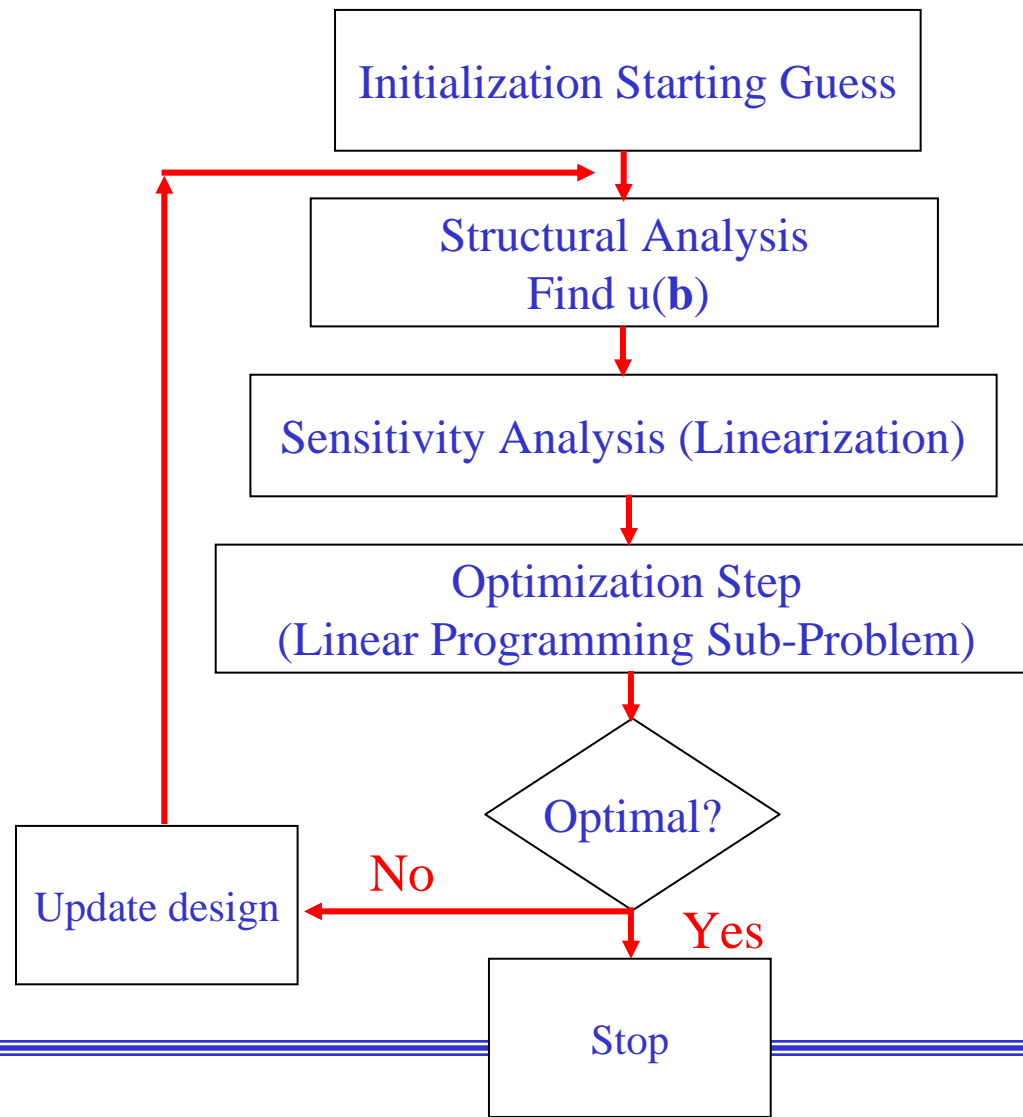
- **PRELIMINARY IDEAS**
  - **The analysis/design framework**
    - **How to capture all possible structural forms?**
  - **Model sparsity issue**
  - **Problem size reduction**
- **APPLICATIONS**
  - **Design for minimal compliance**
  - **Design for stability**



# The Analysis and Design Framework



# Gradient-Based Optimization Algorithm



# Sparsity of Long-Span Bridges



Sunshine Skyway bridge cable-stayed bridge in Tampa, Florida



Akashi Bridge suspension bridge In Japan

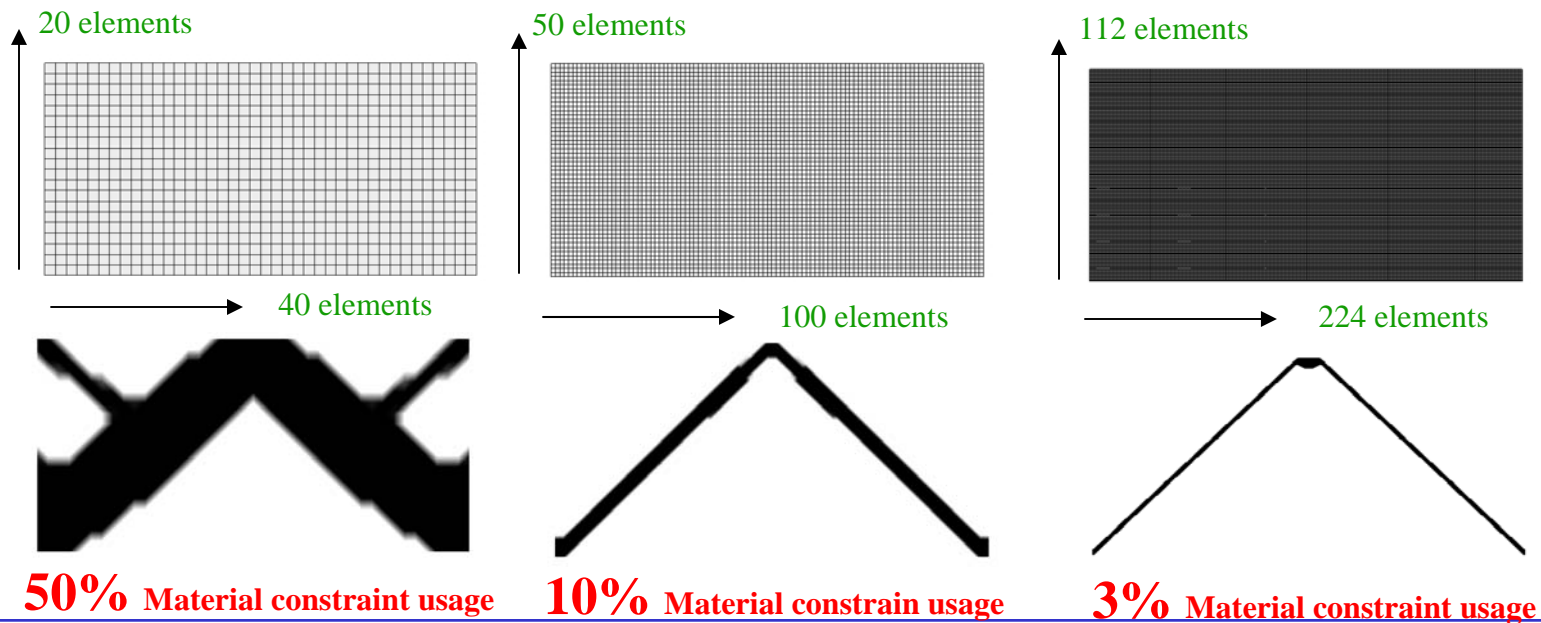


Most long-span bridges occupy  $< 1\%$  of their envelope volume.



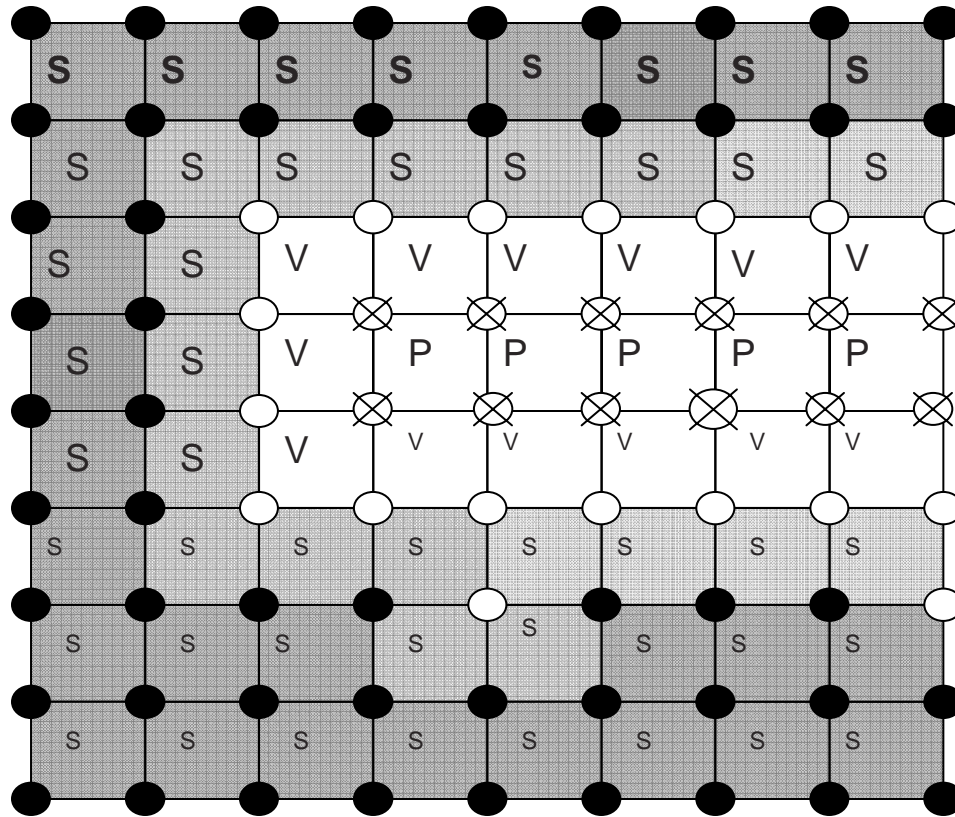
# Capturing SPARSITY in Continuum Topology Optimization

- Fixed-mesh model of full envelope volume;
  - must capture the form of the structure with realistic sparsity
  - must capture mechanical performance of the structure
- Fine meshes are required;
- Implies large computational expense;





# Analysis Problem Size Reduction



Nodes having vanishing design variable values are denoted with open circles; filled circles denote nodes associated with nonzero design values; nodes represented by open circles with X's are "prime" nodes whose degrees of freedom are restrained in the size reduction method. Elements designated with "S" are at least partially solid and those with "V" are devoid of material. Those designated with "P" are prime and need not be considered during structural analysis since all of their degrees of freedom are restrained.



## Strategy for Computational Efficiency:

- 1. Start with coarse structural model**
- 2. Find associated optimal arrangement of material;**
- 3. Construct a finer structural model;**
- 4. Map the current design onto the finer structural model;**
- 5. Reduce allowable material usage**
- 6. Return to step #2.**



- Structure modeled as linearly elastic system

- Stability analysis performed via linearized buckling analysis

- Linear elastic problem:  $\mathbf{K}_L \cdot \mathbf{u} = \mathbf{f}^{ext}$       Compliance:  $\Pi = \frac{1}{2} \mathbf{f}^{ext} \cdot \mathbf{u} = \frac{1}{2} \mathbf{u} \cdot \mathbf{K}_L \cdot \mathbf{u}$

- Eigenvalue problem:  $\mathbf{K}_L(b)\psi + \lambda \mathbf{G}(u, b)\psi = 0$        $\lambda = -\frac{\psi^T \mathbf{K}_L \psi}{\psi^T \mathbf{G} \psi}$

- Objective function:  $L(u, b) = \frac{1}{\min(\lambda)}$

- Design sensitivity analysis:

$$\frac{dL}{db} = -\psi^T \left( \frac{\partial \mathbf{G}}{\partial b} + \frac{1}{\lambda} \frac{\partial \mathbf{K}_L}{\partial b} \right) \psi + (u^a)^T \left( \frac{\partial \mathbf{K}_L}{\partial b} \cdot u - \frac{\partial \mathbf{f}^{ext}}{\partial b} \right)$$

$$\mathbf{K}_L u^a = \psi^T \frac{\partial \mathbf{G}}{\partial u} \psi$$



## Design Constraints

- Bounds on individual design variables

$$0 \leq \phi_j \leq 1 \quad J=1,2,\dots,\text{NUMNP}$$

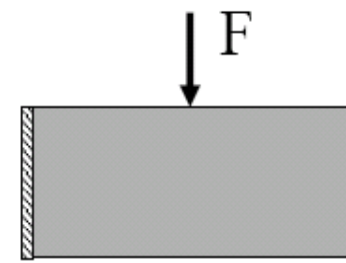
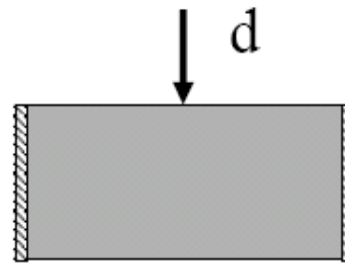
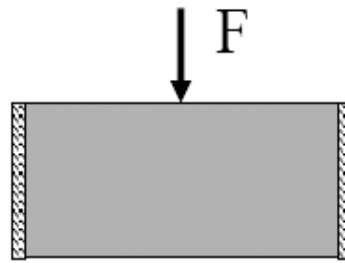
- Material usage constraint

$$\frac{\int_{\Omega} \phi(\mathbf{X}) \, d\Omega}{\int_{\Omega} d\Omega} \leq C$$

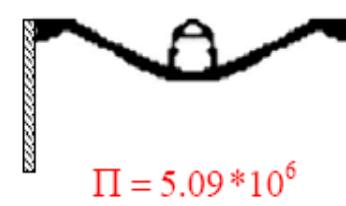
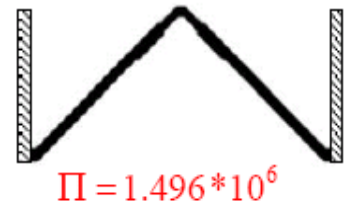


# Fixed-End Beam Problem, Design for Stability

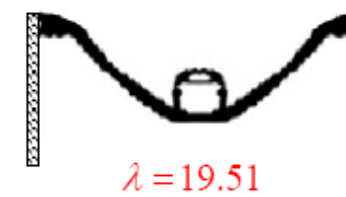
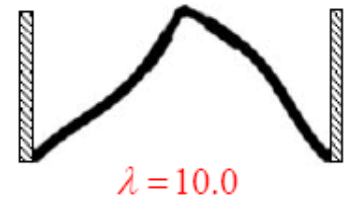
Design domain with 10% material constraint.



Resulting topology



First buckling mode.



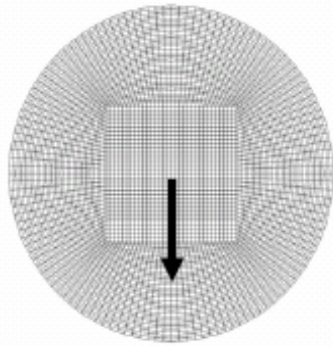
Minimizing the general compliance.

Maximizing the min. critical buckling load, nonlinear formulation.

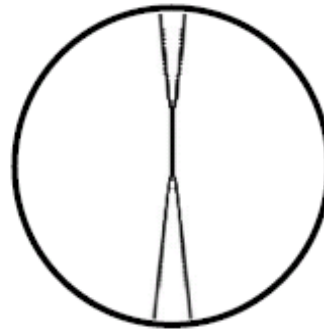
Maximizing the min. critical buckling load, linearized buckling.



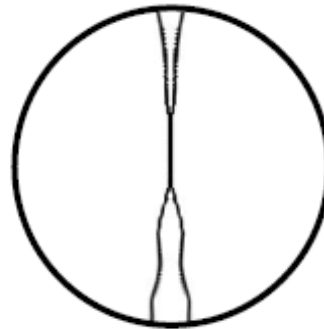
# Circle Problem



Design domain  
With material  
2.5% Constraint

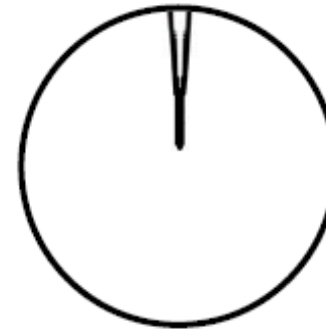


$$\Pi = 4.09 \cdot 10^5$$

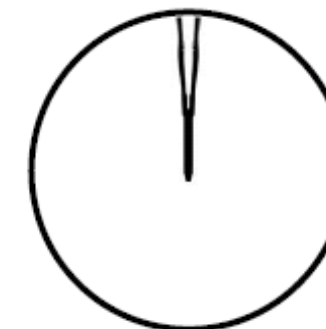


$$\lambda = 1.57 \cdot 10^9$$

Minimizing structural  
Compliance



$$\Pi = 2.66 \cdot 10^5$$



$$\lambda = 3.28 \cdot 10^9$$

Maximizing the min.  
critical buckling load,  
linearized buckling.

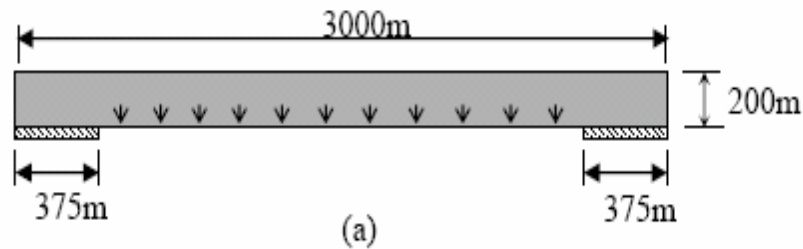
Undeformed  
configuration

Deformed  
configuration



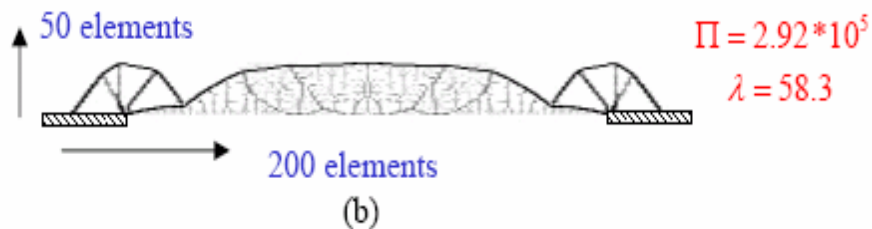
# Long-Span Bridge Problem "2-Supports"

Design domain with  
12.5% material constraint  
and 10 kPa applied load.

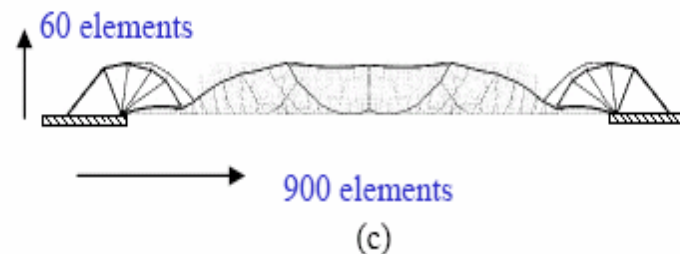


50 elements

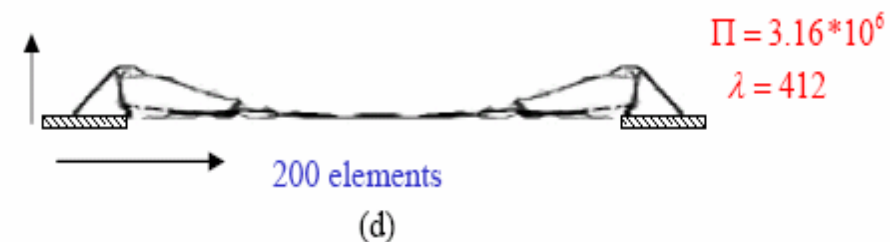
Compliance Minimization



Compliance Minimization  
(finer mesh)

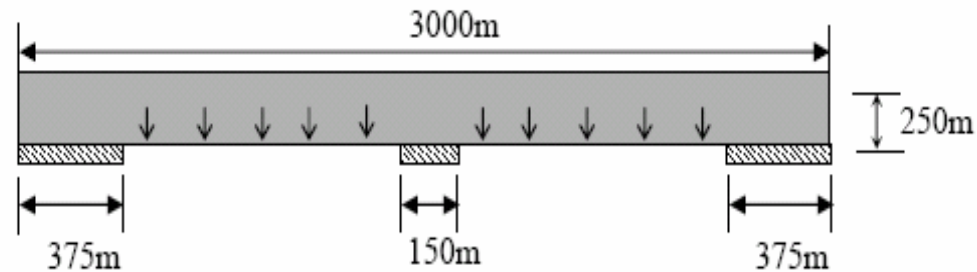


Optimum linearized buckling  
stability

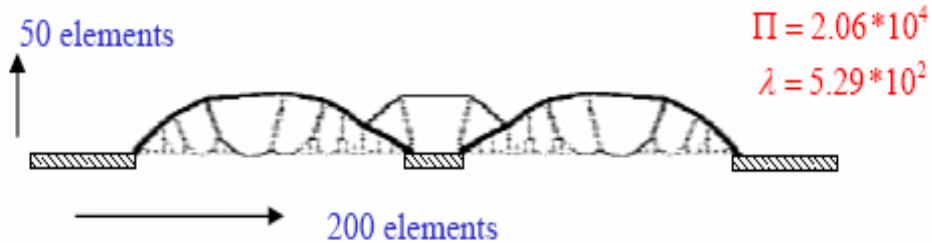


# Long-Span Bridge Problem "3-Supports"

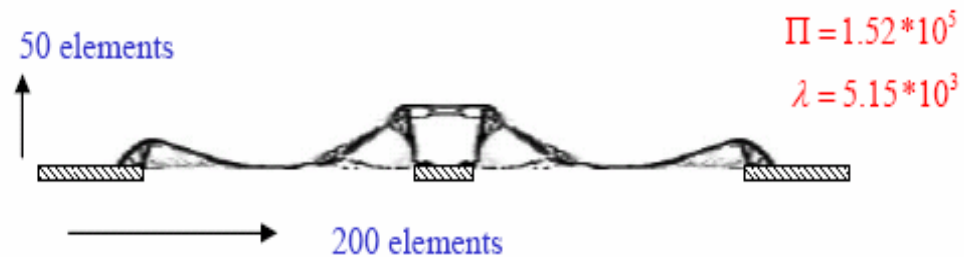
Design domain with  
12.5% material constraint  
and 10 kPa applied load.



Compliance Minimization



Optimum linearized buckling stability





## Current Research Issues:

- ❑ **Designing Structures in 3-Dimensions**
  - ❑ **Iterative Eigensolvers**
    - ❑ **How to reliably solve:  $K\phi + \lambda G\phi = 0$  for the minimum eigenpair without ever forming or factorizing K or G?**
- ❑ **Design of Compliant Mechanisms**
  - ❑ **Micro-devices**

